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GEOTECHNICAL
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GCI PROJECT No. 25-G-30380

Subsurface Exploration and Geotechnical Engineering Report

Gateway Crossing Retail Development
Weldon Road and Faulk Road
Jerome Township, Ohio

Prepared for:
K2 Retail Construction and Development

September 5, 2025



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September 5, 2025

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**Reference: Subsurface Exploration and Geotechnical Engineering Report
Gateway Crossing Retail Development
Weldon and Faulk Roads – Jerome Township, Ohio
GCI Project No. 25-G-30380**

As you authorized, Geotechnical Consultants, Inc. (GCI) performed a subsurface exploration and prepared a geotechnical engineering report for the above referenced project. In summary, our borings encountered a surface topsoil (0.5 feet to 1.1 feet), underlain by stained to brown mottled gray lean clay and glacial till. Sand and gravel lenses were common. We did not encounter bedrock within the depths of the borings. We encountered groundwater seepage in six of the borings at depths of 4 to 17 feet.

Geotechnical considerations for the project include proper site stripping, subgrade stability for soft or heavily stained lean clay soils, new fill placement and compaction (as needed), and proper installation of foundations, slabs, and pavements. Removal of the structures along the west edge of the site is also a consideration. Provided these issues are properly addressed, it is GCI's opinion that this site is suitable for support of the development using conventional shallow foundations, concrete slab-on-grade, and flexible or rigid pavements. We discuss geotechnical considerations and provide foundation recommendations in more detail within the report.

After you have reviewed the report, feel free to contact us with any questions you may have. We appreciate the opportunity to provide our services for this project and hope to continue providing our services through construction.

Respectfully submitted,
Geotechnical Consultants, Inc.

Ryan D. Folsom, P.E.
Manager of Engineering Services



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In-House Reviewer

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INTRODUCTION

As authorized by Paul Evans on behalf of K2 Retail Construction and Development, Geotechnical Consultants, Inc. (GCI) performed a subsurface exploration and prepared this geotechnical engineering report for the proposed “ Gateway Crossing” commercial and retail development located in Jerome Township, Ohio. Prior to drilling, GCI was provided with a development plan showing the proposed building, pavement, and pond layout (prepared by K2 and dated November 27, 2024).

Our subsurface study consisted of twenty-six (26) standard penetration borings drilled across the proposed development area in a general grid pattern. We located the boings in the field using the provided site plan, GIS coordinates, and handheld GPS devices; locations should be considered approximate. We attach the test boring logs and a plan showing the approximate boring locations in the appendix.

The intent of this study was to evaluate subsurface conditions and offer geotechnical recommendations relative to site preparation, earthwork, foundations, slabs, and pavements for the proposed development. We issue this report prior to the receipt of final site development and grading plans. GCI should review these plans when available, and provide additional borings and recommendations, if necessary.

We prepared this report for the exclusive use of K2 Retail Construction and Development and their consultants/client for specific application to the above referenced project in Jerome Township, Ohio in accordance with generally accepted soil and foundation engineering practices. We make no warranty, expressed or implied.

SITE AND PROJECT DESCRIPTIONS

The project site is an approximate L-shaped piece of land located at the intersection of Weldon/Faulk Road and State Route 161 in Jerome Township, Union County, Ohio. The proposed development is located adjacent to an existing retail complex. We show the general site location on an attached map.

At the time of drilling, the property was mostly an open farm field with tree lines and scrub along the property lines. Residential structures were located along Weldon/Faulk and SR-161. Topographically, the site was relatively flat and level with less than 5 feet of estimated relief. The aerial image below shows conditions at the site generally similar to those encountered during our drilling operations.



Aerial Photograph of Site and Approximate Boundary (Google Earth)

We understand that the project will include a primary $\pm 150,000$ square foot retail center with several smaller retail/restaurant outlots. The development will include ponds and roadways. A large section in the north of the property did not show proposed

development but included a detention pond. Based on the existing and surrounding topography we have assumed that cuts and fills at the site will be minor and on the order of 5 feet to create the building pad(s)

SUBSURFACE CONDITIONS

GCI mobilized truck-mounted rotary drill rigs (with automatic sampling hammers) to the site between August 6th, and 12th, 2025. We drilled twenty-six (26) standard penetration borings to depths of 10, 15, or 20 feet at the approximate locations included in the boring location plan.

We have attached boring logs and a boring location plan to the appendix of this report.

We summarize our subsurface findings in the attached table and in the discussion below.

Refer to individual boring logs for more detailed subsurface information at specific boring locations.

Surface Cover

The borings encountered surface topsoil ranging in thickness from 0.5 feet to 1.1 feet.

Natural Soils

Below the surface cover, the borings encountered moderate to high plasticity, brown and brown mottled gray lean clay (classified as CL under the Unified Soil Classification System). The upper portions of these soils were locally stained due to water filtering down through the overlying topsoil. In boring P-4, the staining was noted up to 3 feet and consisted of a black, high plasticity, fat clay (CH). Standard penetration testing indicated the lean clay was soft to medium stiff in cohesive consistency.

Below the mottled lean clay, we encountered low to moderate plasticity brown glacial till, which was visually classified as sandy lean clay to sandy lean clay with gravel (CL). The till graded to a gray color in nineteen of the borings at depths of 9 feet to 16 feet below existing grade. The gray till was typically classified as sandy lean clay to sandy lean clay with gravel (CL). We noted occasional thin silty sand and gravel layers and cobbles within the till soils. Standard penetration testing indicated the till soils were generally medium stiff to very stiff in cohesive consistency. Some hard zones were noted with depth in the glacial till.

Seven of the borings encountered thicker deposits of silt and sand at depths of 9 to 17 feet. These strata were classified as mixtures of silty sand (SM) and sandy silt (ML) or silty sand silty gravel (SM). Standard penetration testing in the sand and silt layers indicated medium dense to dense conditions.

The borings terminated in the brown or gray glacial till (or associated sand/layers) at depths of 10, 15, or 20 feet.

Bedrock

We did not encounter bedrock within the depths of the borings; maximum of 20 feet.

Groundwater and Soil Moisture Conditions

We encountered groundwater seepage in six of the borings at depths of 4 to 17 feet. Upon completion, the groundwater generally remained within a few feet of originally encountered levels. We expect that this water is associated with the granular seams of

the glacial till soils. The water encountered at 4 feet in boring B-11 is likely perched since a secondary water level was encountered at 11 feet.

We generally noted the recovered samples as moist and very moist, with occasional wet samples noted near, and below, the encountered groundwater seepage. Some near-surface samples were noted as damp/dry due to the prevailing dry summer climate at the time of drilling. Note that soil moisture conditions and groundwater levels fluctuate in response to precipitation events, seasonal changes, stabilization time, and other factors that may differ from the time of our measurements.

ANALYSES AND CONCLUSIONS

GEOTECHNICAL EVALUATION

It is GCI's opinion that with proper preparation procedures, the site is generally suitable for the anticipated development using conventional shallow spread footings, continuous wall foundations, and concrete slabs-on-grade. Standard flexible or rigid pavement sections can also be used for the development provided the soil subgrades are firm and stable prior to proceeding with pavement construction. We discuss geotechnical considerations in the following paragraphs.

Site Stripping

We recommend any surface organics, including topsoil, vegetation, stumps from removed trees, etc., be completely removed to expose stable existing natural soils prior to placing new fill, underslab aggregate, or pavement base aggregate. Stripping should extend to a minimum of 5 feet laterally beyond proposed building and pavement areas. Topsoil and

organic matter can be stockpiled for reuse in landscaping mounds, redistributed in proposed green space areas, or disposed at an off-site location.

If drain tile is encountered, it should be removed from the construction area plus to 10 feet beyond. It should be allowed to drain outside these limits but should be diverted so as to not impact the development. Excavations should be backfilled with structural fill.

Existing Construction

We note the presence of residential structures along Weldon/Faulk Road and State Route 161. GCI recommends that remnants of all prior/existing structures within the proposed development be completely removed, including existing at-grade slabs, basement slabs, walls, foundations, below-grade structure elements, pavements, and subsurface utilities (including field tiles and trench backfill) to a minimum of 10 feet laterally outside the building, pavement and pond limits. All existing utilities should be removed and rerouted from below the proposed building footprint, plus 10 feet laterally. Abandoned utility lines (outside the zone of influence of proposed construction) should be capped to prevent loss of soil.

Subgrade Stability

We encountered stained and soft to medium stiff conditions in the upper zones of the soil profile. Portions of the stained materials can be left in place provided they are stable and are low in organic content.

The earthwork contractor should proof-roll the exposed soil subgrade using a fully-loaded, tandem-axle dump truck (or equivalent) to identify potential soft, yielding

subgrade areas. Soft spots identified during the proof-roll should be undercut to firm, stable conditions or otherwise stabilized prior to placing additional fill, slab construction, or paving. Structural fill can be placed to design grade provided the exposed subgrades are proof-rolled, and firm, stable conditions are verified prior to fill placement.

Stabilization of soft subgrades by disking, aerating/drying, and re-compaction may be feasible during traditionally drier times of the year. During wet seasons, partial undercutting and replacing of wet soils with structural fill, drying with soil additives such as lime, or use of geosynthetics may be needed to create a stable subgrade before placing controlled fills. The use of soil additives such as lime and flyash or installation of geosynthetics should be reviewed by our office prior to use in the field. We advise careful routing of construction traffic to help minimize instabilities of near-surface soils during wet seasons.

The severity of soft, very moist subgrade conditions will depend on the time of year earthwork is performed, and the amount of moisture within the subgrade soils. We expect fewer problems with soft and wet subgrades if earthwork and mass grading operations are performed during traditionally drier times of the year (i.e. late spring, summer, and early fall). Traditional wetter seasons (i.e. late fall, winter, and early spring) will contribute to problems associated with soft, very moist subgrades.

New Fill Placement and Compaction

Structural fill can be placed to design grade once the subgrades are brought to firm and stable conditions. Non-organic, clay-based site soils are suitable for reuse in new, controlled fills provided proper moisture control is maintained. Depending on the time of year of earthwork, the fill materials may require drying to achieve compaction. Fill

materials within building pads and pavement areas should be placed in a controlled manner as described in the *Site Preparation and Earthwork* section of this report.

FOUNDATIONS

Foundations bearing on firm and stable natural soils or new fill placed directly on firm and stable natural soils can be designed using a maximum allowable bearing capacity of 3,000 pounds per square foot.

Regardless of calculated sizes, GCI recommends minimum footing sizes of 30 inches square for columns and 16 inches in width for strip footings to eliminate a potential punching effect. All exterior foundations should extend to local frost bearing depth (32 inches) or to acceptable soils, whichever is deeper. Interior footings in heated areas may be placed as shallow as feasible as long as they are bearing on acceptable soils. Typical to local practice, if soft, unstable, or unsuitable soils are encountered at footing subgrade, undercut to stable soils. *Soft, unstable footing subgrades should be reviewed by the soils engineer prior to undercut.* Undercut areas can be backfilled to footing subgrade using a controlled density fill (CDF), such as K-Krete, to allow footing construction at design grade.

FLOOR SLABS

Once the building pads have been prepared, conventional concrete slab-on-grade is feasible for the anticipated construction. Subgrades should be thoroughly proof-rolled and any soft, yielding areas brought to a stable condition prior to slab construction or placement of aggregate base. GCI recommends placing a minimum of 4 inches of granular fill (well-graded crushed stone, such as AASHTO #57 Stone or ODOT Item 304)

under the floor slabs to serve as a capillary cut-off, and to provide a uniform, firm base. The aggregate thickness should be increased to 6 inches in more heavily loaded areas.

We recommend placing a vapor retarder below the slab where moisture may be a problem with slab-on-grade floor coverings.

SEISMIC FACTOR

The borings at the site revealed a typical subsurface profile consisting of soft to medium stiff lean clay, and stiff glacial till soils with interbedded sand and silt strata. In accordance with the Ohio Building Code, the site is estimated as a Site Class D – stiff soil profile.

EXCAVATIONS

The site soils (clay-based and granular) can be excavated with conventional track hoe equipment. We did not encounter bedrock and do not expect that bedrock will be encountered within the anticipated excavation depths. Sidewall stability may be an issue where excavations into any saturated sand and gravel layers/lenses (found within the glacial till soils). Excavations that encounter these granular layers may need to be laid back or braced to maintain sidewall stability. **All site excavations should comply with current OSHA regulations.**

GROUNDWATER

We encountered groundwater seepage in six of the borings at depths of 4 to 17 feet below grade. We do not anticipate that groundwater will pose significant problems with shallow foundation and shallow utility trench excavations associated with the proposed

development. If water is encountered in site excavations, the excavations should be dewatered to allow footing construction and utility trench backfilling in dry conditions. We expect groundwater seepage flows in excavations can be handled with portable sump pumps and working mats of crushed stone, as needed. Contact GCI for additional recommendations if excessive groundwater conditions are encountered.

BASINS

Several borings were performed in the areas of the proposed ponds/basins. In general, these borings encountered primarily clay-based soils. The natural lean clays will provide a suitable liner for the ponds. However, granular layers would provide a path for the pond to leak. If the basins are to maintain a normal pond level, a liner will be needed where granular seams are encountered in the pond bottoms and sides. To provide the pond liner, we recommend that granular layers in the pond bottoms and sides be gouged out and covered with a minimum of 18 inches of compacted clay. Fill should be properly placed and compacted to a minimum of 98% Standard Proctor maximum dry density, as described in the *Site Preparation and Earthwork* section of this report. Any dewatering to construct the liner should be continued until the pond has been filled.

PAVEMENTS

Provided the site is properly prepared, conventional aggregate base and flexible asphalt wearing course pavements can be used. Prior to pavement construction, the subgrade should be carefully proof-rolled, and stabilized as necessary to provide a CBR value of at least 3. A specific pavement design is beyond the scope of work of this report; however, minimum recommended pavement sections are provided below. A site-specific pavement design would require additional laboratory testing and pavement use criteria.

Properly compacted, GCI anticipates the site soils would have a CBR value of at least 3. Based on the soils encountered, and assuming properly prepared subgrades, GCI recommends a minimum light-duty pavement section consisting of 3 inches of asphalt over 8 inches of aggregate base. For heavy-duty traffic areas, including the main traffic aisles and areas subjected to refuse truck traffic, GCI suggests a pavement section consisting of a minimum of 4 inches of asphalt over 10 inches of aggregate base. GCI recommends a minimum of 8 inches of air-entrained, Portland cement concrete for any dumpster pad(s) and dumpster pad approach area(s).

Providing adequate subbase drainage is important to future pavement performance. Finger drains connecting to weep-holes in inlets, proper grading of pavement subgrades and surfaces to shed run-off, and underdrains in pavement swales are suggested subbase drainage methods and should be designed by the site civil engineer. Installing a medium-duty geogrid below the base aggregate course in areas subjected to stopping and turning traffic or concentrated traffic flow, such as the main entrance/exit drives, will increase the structural number of the pavement section and improve the pavement performance.

SITE PREPARATION AND EARTHWORK

As a general approach to the site preparation, we recommend the following:

1. Strip existing vegetation, topsoil, stumps from removed trees, and root mat systems from beneath proposed construction areas plus 5 feet laterally beyond. Topsoil can be stockpiled for later distribution in site landscaping mounds, green space areas, used to fill borrow pits, or disposed of off-site. Remove clay tile if encountered in excavations, ensuring the tile is allowed to drain properly outside the development area.
2. Remove existing construction as detailed in the report.
3. Carefully proof-roll the soil subgrades using a fully-loaded, tandem-axle dump truck (or equivalent) to identify potential soft, yielding subgrade areas. Soft/unstable areas identified during the proof-roll should be undercut to stable subgrade conditions or

otherwise stabilized prior to placing controlled fill, placing underslab aggregate, or placing pavement subbase materials.

4. Place controlled fill to design grade within the proposed construction areas, as required. The non-organic site soils are suitable for reuse in controlled fills. Off-site borrow materials should be reviewed by our office prior to use.
5. Place controlled fills in maximum 8-inch thick loose lifts and compact to a minimum of 98% of the maximum Standard Proctor dry density (ASTM D-698). The moisture in the site soils should be controlled to within $\pm 3\%$ of the optimum Standard Proctor moisture content. **Depending on the time of year of earthwork, moisture adjustment of the site soils may be required to achieve proper compaction.** Cohesive soils will compact best with a static-weight sheepsfoot roller. Clay-based soils that are over the optimum moisture content will tend to pump if exposed to vibratory sheepsfoot compaction equipment.
6. Excavate for foundations after fill placement for the building pads is complete. Footings may bear on suitable soils as recommended in the report. Undercut any isolated soft zones to achieve firm bearing conditions. Refer to the *Geotechnical Evaluation and Foundations* section of this report for specific foundation design parameters.
7. Pavement areas should be steel-wheel rolled to a smooth surface prior to placement of base aggregate. Subgrade preparation during wet seasons may require the use of engineering fabric or geogrid.
8. We recommend that GCI be retained to observe proof-rolling operations, cut and fill operations, and footing excavations.
9. If work is performed during the winter (e.g., when freezing temperatures occur), special protective measures will be required during fill placement and footing construction procedures. Contact GCI for additional cold weather recommendations, as needed.

CONSTRUCTION MATERIALS ENGINEERING AND TESTING

GCI provides construction materials engineering and testing services. For project continuity throughout construction, we recommend that GCI be retained to observe, test, and document:

- earthwork procedures (stripping, fill placement, compaction, utility trench backfill, etc.),
- slab preparation (proof-rolling, excavations, undercuts, etc.),
- concrete placement and compressive strength testing (footings, slabs, pavements, etc.), and

- structural steel (welds, bolts, etc.).

The purpose of this work is to assess that the intent of our recommendations is being followed and to make timely changes to our recommendations (as needed) in the event site conditions vary from those encountered in our borings. Please contact our field department to initiate these services.

FINAL

We recommend that GCI review final site layout and grading plans. Recommendations contained in this report may be changed based on review of final site plans. If any changes in the nature, design or locations of the construction are planned, conclusions and recommendations should not be considered valid unless verified in writing by GCI.

The recommendations contained in this report are the opinion of GCI based on the subsurface conditions found in the borings and available development information. This report has been prepared for design purposes only and should not be considered sufficient to prepare an accurate bid document. It should be noted that the nature and extent of variations between borings might not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

If you have any questions or need for any additional information, please contact our office. It has been a pleasure to be of service to you on this project, and we hope to continue our services through construction.



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APPENDIX – Gateway Crossing – Jerome Township

**General Notes for Soil Sampling and Classifications
Site Location Map and Boring Location Plan
Summary of Encountered Subsurface Conditions
Test Boring Logs (26 sheets)**



GENERAL NOTES FOR SOIL SAMPLING AND CLASSIFICATIONS

BORINGS, SAMPLING AND GROUNDWATER OBSERVATIONS:

Drilling and sampling were conducted in accordance with procedures generally recognized and accepted as standard methods of exploration of subsurface conditions. The borings were drilled using a truck-mounted drill rig using auger boring methods with standard penetration testing performed in each boring at intervals ranging from 1.5 to 5.0 feet. The stratification lines on the logs represent the approximate boundary between soil types at that specific location and the transition may be gradual.

Water levels were measured at drill locations under conditions stated on the logs. This data has been reviewed and interpretations made in the text of the report. Fluctuations in the level of the groundwater may occur due to other factors than those present at the time the measurements were made.

The Standard Penetration Test (ASTM-D-1586) is performed by driving a 2.0 inch O.D. split barrel sampler a distance of 18 inches utilizing a 140 pound hammer free falling 30 inches. The number of blows required to drive the sampler each 6 inches of penetration are recorded. The summation of the blows required to drive the sampler for the final 12 inches of penetration is termed the Standard Penetration Resistance (N). Soil density/consistency in terms of the N-value is as follows:

COHESIONLESS DENSITY		COHESIVE CONSISTENCY	
0-10	Loose	0-4	Soft
10-30	Medium Dense	4-8	Medium Stiff
30-50	Dense	8-15	Stiff
50 +	Very Dense	15-30	Very Stiff
		30 +	Hard

SOIL MOISTURE TERMS

Soil Samples obtained during the drilling process are visually characterized for moisture content as follows:

MOISTURE CONTENT	DESCRIPTION
Damp	Soil moisture is much drier than the Atterberg plastic limit (where soils are cohesive) and generally more than 3% below Standard Proctor "optimum" moisture conditions. Soils of this moisture generally require added moisture to achieve proper compaction.
Moist	Soil moisture is near the Atterberg plastic limit (cohesive soils) and generally within $\pm 3\%$ of the Standard Proctor "optimum" moisture content. Little to no moisture conditioning is anticipated to be required to achieve proper compaction and stable subgrades.
Very Moist	Soil moisture conditions are above the Atterberg plastic limit (cohesive soils) and generally greater than 3% above Standard Proctor "optimum" moisture conditions. Drying of the soils to near "optimum" conditions is anticipated to achieve proper compaction and stable subgrades.
Wet	Soils are saturated. Significant drying of soils is anticipated to achieve proper compaction and stable subgrades.

SOIL CLASSIFICATION PROCEDURE:

Soil samples obtained during the drilling process are preserved in plastic bags and visually classified in the laboratory. Select soil samples may be subjected to laboratory testing to determine natural moisture content, gradation, Atterberg limits and unit weight. Soil classifications on logs may be adjusted based on results of laboratory testing.

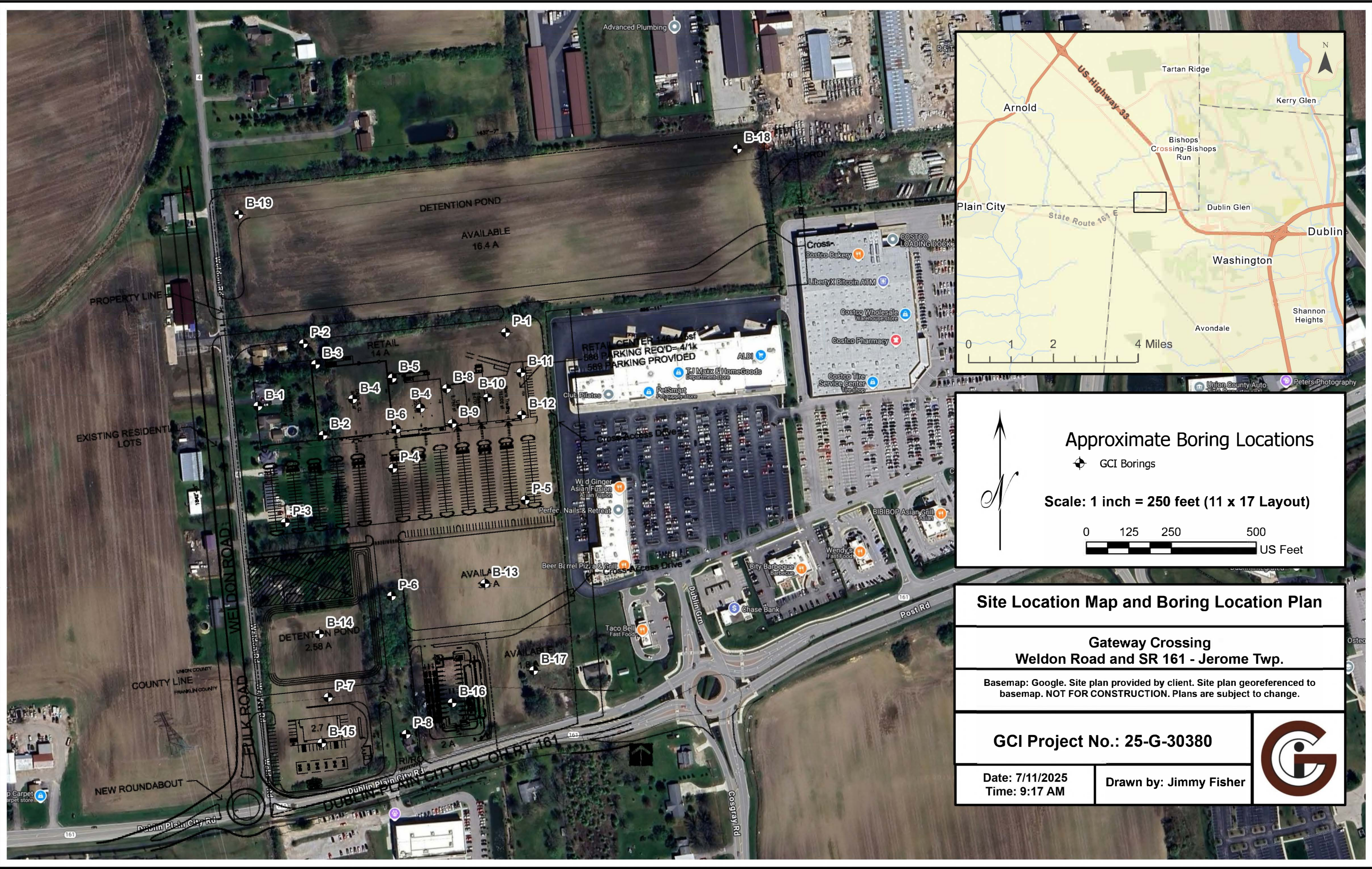
Soils are classified in accordance with the ASTM version of the Unified Soil Classification System. ASTM D-2487 "Classification of Soils for Engineering Purposes (Unified Soil Classification System) describes a system for classifying soils based on laboratory testing. ASTM D-2488 "Description and Identification of Soil (Visual-Manual Procedure) describes a system for classifying soils based on visual examination and manual tests.

Soil classifications are based on the following tables (see reverse side):


GENERAL NOTES FOR SOIL SAMPLING AND CLASSIFICATIONS

PARTICLE SIZE DEFINITION		CONSTITUENT MODIFIERS	
Boulders:	>12"		
Cobbles:	3" to 12"	Trace	Less than 5%
Gravel:	Coarse: 3/4" to 3"	Few	5-10%
	Fine: No. 4 (3/16") to 3/4"	Little	15-25%
Sand:	Coarse No. 10 (2.0mm) to No. 4 (4.75mm)	Some	30-45%
	Medium No. 40 (0.425mm) to No. 10 (2.0mm)	Mostly	50-100%
	Fine No. 200 (0.074mm) to No. 40 (0.425mm)		
Silt & Clay	<0.074mm; classification based on overall plasticity; in general clay particles <0.005mm.		


ASTM/UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART		
COARSE-GRAINED SOILS (more than 50% of materials is larger than No. 200 sieve size)		
GRAVELS More than 50% of coarse fraction larger than No. 4 sieve size	<i>Clean Gravel (less than 5% fines)</i>	
	GW	Well-graded gravel, gravel-sand mixtures, little or no fines
	GP	Poorly-graded gravels, gravel sand mixtures, little or no fines
	<i>Gravels with fines (more than 12% fines)</i>	
	GM	Silty gravels, gravel-sand-silt mixtures
	GC	Clayey gravels, gravel-sand-clay mixtures
SANDS More than 50% of coarse fraction smaller than No. 4 sieve size	<i>Clean Sands (Less than 5% fines)</i>	
	SW	Well-graded sands, gravelly sands, little or no fines
	SP	Poorly-graded sands, gravelly sands, little or no fines
	<i>Sands with fines (More than 12% fines)</i>	
	SM	Silty sands, sand-silt mixtures
	SC	Clayey sands, sand-clay mixtures
Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows: Less than 5 percentGW, GP, SW, SP Greater than 12 percentGM, GC, SM, SC 5 to 12 percentBorderline cases requiring dual symbols: SP-SM, GP-GM, etc.		
FINE-GRAINED SOILS (50% or more of material is smaller than No. 200 sieve size)		
SILTS AND CLAYS Liquid Limit less than 50%	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
	CL	Inorganic clays or low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
	CL-ML	Inorganic silty clay of slight plasticity, P.I. between 4 and 7
	OL	Organic silts and organic silty clays of low plasticity
SILTS AND CLAYS Liquid Limit 50% or greater	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
	CH	Inorganic clays of high plasticity, fat clays
	OH	Organic clays or medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS	PT	Peat and other highly organic soils



Approximate Boring Locations

 GCI Borings


Scale: 1 inch = 250 feet (11 x 17 Layout)



Site Location Map and Boring Location Plan

**Gateway Crossing
Weldon Road and SR 161 - Jerome Twp.**

Basemap: Google. Site plan provided by client. Site plan georeferenced to basemap. NOT FOR CONSTRUCTION. Plans are subject to change.

GCI Project No.: 25-G-30380		
<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 50%; text-align: center;">Date: 7/11/2025 Time: 9:17 AM</td> <td style="width: 50%; text-align: center;">Drawn by: Jimmy Fisher</td> </tr> </table>		Date: 7/11/2025 Time: 9:17 AM
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Summary of Encountered Subsurface Conditions

Gateway Crossing Retail Development
Weldon and Faulk Roads - Jerome Twp., Ohio
GCI Job Number: 25-G-30380

Borehole	Surface Layer	Topsoil Thickness (ft.)	Groundwater: Level Encountered (ft)	Groundwater: Level at Completion (ft)	Depth to Top of Lean Clay (ft)	Depth to Top of Brown Till (ft)	Depth to Top of Gray Till (ft)	Depth to Top of Sand/Gravel (ft)	Bottom of Boring Depth (ft)
			Depth	Depth					
B-1	Topsoil	0.8	--	--	0.8	4.0	12.0		15.0
B-2	Topsoil	0.6	--	--	0.6	6.0	12.0		20.0
B-3	Topsoil	0.9	--	--	0.9	6.5	11.5		15.0
B-4	Topsoil	1.1	--	--	1.1	5.0	13.0		20.0
B-5	Topsoil	0.8	14	10	0.8	5.0	11.0	14.0	15.0
B-6	Topsoil	0.8	12	11	0.8	5.0	16.0	12.0	20.0
B-7	Topsoil	0.7	--	--	0.7	4.0	14.0	12.0	15.0
B-8	Topsoil	1.0	17	19	1.0	3.5	11.0	17.0	20.0
B-9	Topsoil	0.8	--	--	0.8	4.5	12.0	9.0	15.0
B-10	Topsoil	0.7	--	--	0.7	3.0	10.0		20.0
B-11	Topsoil	0.9	4	11	0.9	6.5	--	11.0	15.0
B-12	Topsoil	0.7	17	19	0.7	4.0	13.0	17.0	20.0
B-13	Topsoil	0.6	--	--	0.6	5.0	13.0		15.0
B-14	Topsoil	0.9	--	--	0.9	6.5	12.0		20.0
B-15	Topsoil	0.8	--	--	0.8	4.0	12.0		15.0
B-16	Topsoil	0.7	--	--	0.7	4.0	9.0		15.0
B-17	Topsoil	0.5	--	--	0.5	3.0	12.0		15.0
B-18	Topsoil	0.7	9	8	0.7	5.0	12.0		15.0
B-19	Topsoil	0.6	--	--	0.6	5.5	13.0		15.0
P-1	Topsoil	1.1	--	--	1.1	4.0	9.0		10.0
P-3	Topsoil	0.8	--	--	0.8	7.0	--		10.0
P-4	Topsoil	1.1	--	--	3.0	7.0	--		10.0
P-5	Topsoil	0.6	--	--	0.6	6.0	--		10.0
P-6	Topsoil	0.8	--	--	0.8	4.5	--		10.0
P-7	Topsoil	0.9	--	--	0.9	4.0	--		10.0
P-8	Topsoil	0.8	--	--	0.8	7.0	--		10.0

Average Topsoil Depth at boring locations: 0.8 feet



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-1
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/12/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
None FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION						
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness						
				0-6	6-12	12-18										
	--	0.0-1.5	SS	3	2	3	Damp	0.8	▲▲▲▲▲▲▲▲▲▲	Topsoil						
									▨▨▨▨▨▨▨▨▨▨	Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand						
	3.0	2.0-3.5	SS	3	2	2	Moist									
								4.0	▨▨▨▨▨▨▨▨▨▨							
	4.0	4.0-5.5	SS	3	4	6	Moist			Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel						
5									▨▨▨▨▨▨▨▨▨▨							
	4.0	8.5-10.0	SS	8	10	16	Moist									
10																
								12.0	▨▨▨▨▨▨▨▨▨▨							
										Gray Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel						
	4.5+	13.5-15.0	SS	10	11	10	Moist									
15								15.0	▨▨▨▨▨▨▨▨▨▨							

										BOTTOM OF BORING: 15 feet						

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-2
 CLIENT K2 Retail Construction and Development PROJ. _____ SURF. ELEV. _____
 NO. 25-G-30380 DATE DRILLED 8/12/2025

GROUND WATER OBSERVATION <u>None</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Proportions Used Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	140 lb Wt. x 30" fall on 2" O.D. Sampler <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="border-right: 1px solid black;">Cohesionless Density</td> <td>Cohesive Consistency</td> </tr> <tr> <td style="border-right: 1px solid black;">0 - 10 Loose</td> <td>0 - 4 Soft</td> </tr> <tr> <td style="border-right: 1px solid black;">10 - 30 Medium Dense</td> <td>4 - 8 Medium Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">30 - 50 Dense</td> <td>8 - 15 Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">50 + Very Dense</td> <td>15 - 30 Very Stiff</td> </tr> <tr> <td></td> <td>30 + Hard</td> </tr> </table>	Cohesionless Density	Cohesive Consistency	0 - 10 Loose	0 - 4 Soft	10 - 30 Medium Dense	4 - 8 Medium Stiff	30 - 50 Dense	8 - 15 Stiff	50 + Very Dense	15 - 30 Very Stiff		30 + Hard
Cohesionless Density	Cohesive Consistency													
0 - 10 Loose	0 - 4 Soft													
10 - 30 Medium Dense	4 - 8 Medium Stiff													
30 - 50 Dense	8 - 15 Stiff													
50 + Very Dense	15 - 30 Very Stiff													
	30 + Hard													

LOCATION OF BORING See Boring Location Plan

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION						
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness						
				0-6	6-12	12-18										
	--	0.0-1.5	SS	4	5	4	Damp	0.6	Topsoil							
									Brown to Brown Mottled Gray Lean Clay (CL) - moderate plasticity, few sand, trace gravel; silty seams							
	2.0	2.0-3.5	SS	4	3	4	Moist									
	--	4.0-5.5	SS	3	3	3	Moist									
5								6.0	Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel							
	--	8.5-10.0	SS	5	6	8	Moist									
10								12.0	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel							
	4.5+	13.5-15.0	SS	7	7	9	Moist									
15																
	4.5+	18.5-20.0	SS	10	8	9	Moist									
								20.0	BOTTOM OF BORING: 20 feet							

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-3
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/12/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
<u>None</u> FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION						
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness						
				0-6	6-12	12-18										
	--	0.0-1.5	SS	3	4	3	Moist	0.9	Topsoil							
									Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand							
	--	2.0-3.5	SS	3	4	4	Damp									
	4.0	4.0-5.5	SS	3	2	3	Moist		Gray fat clay (CH) veining							
5								6.5								
	4.5+	8.5-10.0	SS	9	12	8	Moist		Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel							
10								11.5								
	4.5+	13.5-15.0	SS	7	9	8	Damp		Gray Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel							
15								15.0								
									BOTTOM OF BORING: 15 feet							

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-5
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/6/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
<u>10.0</u> FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness
				0-6	6-12	12-18				
	3.0	0.0-1.5	SS	3	3	2	Moist	0.8	Topsoil	
									Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand	
	3.5	2.0-3.5	SS	2	2	2	Moist			
									Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel	
	4.5+	4.0-5.5	SS	3	2	4	Moist	5.0		
5									Gray Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel	
	4.5+	8.5-10.0	SS	6	8	9	Moist	11.0		
10									Water Seepage at 14 feet	
	--	13.5-15.0	SS	12	14	12	Very Moist to Wet	14.0		
15								15.0	Brown to Gray Silty Sand (SM) - mostly fine to medium sand, little to some silt, trace gravel	
									BOTTOM OF BORING: 15 feet	

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-6

PROJ. _____ SURF. ELEV. _____

CLIENT K2 Retail Construction and Development NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION				Proportions Used			140 lb Wt. x 30" fall on 2" O.D. Sampler				
11.0 FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS				Trace	Less than 5%		Cohesionless Density		Cohesive Consistency		
				Few	5 to 10%		0 - 10	Loose	0 - 4	Soft	
				Little	15 to 25%		10 - 30	Medium Dense	4 - 8	Medium Stiff	
				Some	30 to 45%		30 - 50	Dense	8 - 15	Stiff	
				Mostly	50 to 100%		50 +	Very Dense	15 - 30	Very Stiff	
									30 +	Hard	
LOCATION OF BORING See Boring Location Plan											
DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*	SOIL IDENTIFICATION Remarks include color, type of soil, etc. Rock-color, type, condition, hardness		
				0-6	6-12	12-18					
	--	0.0-1.5	SS	2	3	2	Damp	0.8	Topsoil		
									Brown to Brown Mottled Gray Lean Clay (CL) - moderate plasticity, few sand, trace gravel; silty seams		
	2.5	2.0-3.5	SS	3	3	3	Moist				
									Brown Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel		
	4.5	4.0-5.5	SS	3	7	6	Moist to Very Moist	5.0			
5									Brown Silty Sand with Gravel (SM) - mostly fine to coarse sand, little silt, little gravel		
	4.5+	8.5-10.0	SS	14	11	14	Moist				
10									Cobbles noted with depth		
	--	13.5-15.0	SS	21	22	16	Moist to Wet	12.0			
									Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel		
								16.0			
15									BOTTOM OF BORING: 20 feet		
	--	18.5-20.0	SS	19	24	27	Wet	20.0			

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-7

PROJ. _____ SURF. ELEV. _____

CLIENT K2 Retail Construction and Development NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION				Proportions Used			140 lb Wt. x 30" fall on 2" O.D. Sampler					
None FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS				Trace	Less than 5%		Cohesionless Density 0 - 10 Loose 10 - 30 Medium Dense 30 - 50 Dense 50 + Very Dense			Cohesive Consistency 0 - 4 Soft 4 - 8 Medium Stiff 8 - 15 Stiff 15 - 30 Very Stiff 30 + Hard		
				Few	5 to 10%							
				Little	15 to 25%							
				Some	30 to 45%							
				Mostly	50 to 100%							
LOCATION OF BORING See Boring Location Plan												
DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*	SOIL IDENTIFICATION Remarks include color, type of soil, etc. Rock-color, type, condition, hardness			
				0-6	6-12	12-18						
	4.0	0.0-1.5	SS	2	3	2	Damp	0.7	Topsoil			
									Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand			
	2.0	2.0-3.5	SS	2	1	2	Moist					
								4.0				
	4.5+	4.0-5.5	SS	8	6	8	Moist		Brown Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel			
5									Cobbles and hard drilling			
	--	8.5-9.8	SS	22	23	50/3	Moist					
10								12.0				
	--	13.5-15.0	SS	18	18	15	Very Moist to Moist	14.0	Brown to Gray Silty Sand with Gravel (SM) - mostly fine to coarse sand, little silt, little gravel			
								15.0	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel			
15									BOTTOM OF BORING: 15 feet			

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-8

CLIENT K2 Retail Construction and Development PROJ. SURF. ELEV. _____
 NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION <u>19.0</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Proportions Used Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	140 lb Wt. x 30" fall on 2" O.D. Sampler <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="border-right: 1px solid black;">Cohesionless Density</td> <td>Cohesive Consistency</td> </tr> <tr> <td style="border-right: 1px solid black;">0 - 10 Loose</td> <td>0 - 4 Soft</td> </tr> <tr> <td style="border-right: 1px solid black;">10 - 30 Medium Dense</td> <td>4 - 8 Medium Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">30 - 50 Dense</td> <td>8 - 15 Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">50 + Very Dense</td> <td>15 - 30 Very Stiff</td> </tr> <tr> <td></td> <td>30 + Hard</td> </tr> </table>	Cohesionless Density	Cohesive Consistency	0 - 10 Loose	0 - 4 Soft	10 - 30 Medium Dense	4 - 8 Medium Stiff	30 - 50 Dense	8 - 15 Stiff	50 + Very Dense	15 - 30 Very Stiff		30 + Hard
Cohesionless Density	Cohesive Consistency													
0 - 10 Loose	0 - 4 Soft													
10 - 30 Medium Dense	4 - 8 Medium Stiff													
30 - 50 Dense	8 - 15 Stiff													
50 + Very Dense	15 - 30 Very Stiff													
	30 + Hard													

LOCATION OF BORING See Boring Location Plan

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION					
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness					
				0-6	6-12	12-18									
	--	0.0-1.5	SS	2	3	3	Damp			Topsoil					
								1.0		Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand					
	3.0	2.0-3.5	SS	2	3	3	Moist								
								3.5		Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel					
	4.5+	4.0-5.5	SS	2	3	5	Moist								
5															
	4.5+	8.5-10.0	SS	10	13	14	Moist								
10															
								11.0		Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel and cobbles fragments					
	4.5+	13.5-15.0	SS	14	9	9	Moist								
15															
								17.0		Water Seepage at 17 feet					
										Gray Silty Sand with Gravel (SM) - mostly fine to coarse sand, little silt, little gravel					
	--	18.5-20.0	SS	4	5	5	Wet								
								20.0		BOTTOM OF BORING: 20 feet					

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-9

PROJ. _____ SURF. ELEV. _____

CLIENT K2 Retail Construction and Development NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
<u>None</u> FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*	SOIL IDENTIFICATION	
				From	To	To			Remarks include color, type of soil, etc. Rock-color, type, condition, hardness	
				0-6	6-12	12-18				
	--	0.0-1.5	SS	2	3	2	Damp	0.8	Topsoil	
									Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand; silty seams	
	1.0	2.0-3.5	SS	2	2	2	Very Moist			
	4.5	4.0-5.5	SS	3	4	5	Moist	4.5		
5									Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel	
	--	8.5-10.0	SS	11	10	12	Moist	9.0		
10									Brown to Gray Silty Sand (SM) - mostly fine to coarse sand, little to some silt, few gravel	
	4.5+	13.5-15.0	SS	19	24	12	Moist	12.0		
15									Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel and cobble fragments	
								15.0		
									BOTTOM OF BORING: 15 feet	

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-10

PROJ. _____ SURF. ELEV. _____

CLIENT K2 Retail Construction and Development NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
None FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION					
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness					
				0-6	6-12	12-18									
	--	0.0-1.5	SS	2	3	3	Damp	0.7	▲▲▲▲▲▲▲▲▲▲	Topsoil					
									▨▨▨▨▨▨▨▨▨▨	Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand; silty seams					
	2.0	2.0-3.5	SS	2	2	2	Damp to Moist	3.0							
									▨▨▨▨▨▨▨▨▨▨	Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel; thin sand and silt layers					
5	--	4.0-5.5	SS	12	7	7	Moist		▨▨▨▨▨▨▨▨▨▨	Frequent cobbles noted					
									▨▨▨▨▨▨▨▨▨▨						
	4.5+	8.5-10.0	SS	10	11	13	Moist	10.0							
10									▨▨▨▨▨▨▨▨▨▨	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel and cobble fragments					
									▨▨▨▨▨▨▨▨▨▨						
	4.5+	13.5-15.0	SS	13	12	11	Moist		▨▨▨▨▨▨▨▨▨▨						
15									▨▨▨▨▨▨▨▨▨▨						
									▨▨▨▨▨▨▨▨▨▨						
	4.5+	18.5-20.0	SS	11	11	19	Moist		▨▨▨▨▨▨▨▨▨▨						
									▨▨▨▨▨▨▨▨▨▨						
								20.0		▨▨▨▨▨▨▨▨▨▨	BOTTOM OF BORING: 20 feet				

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-12

PROJ. _____ SURF. ELEV. _____

CLIENT K2 Retail Construction and Development NO. 25-G-30380 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION				Proportions Used			140 lb Wt. x 30" fall on 2" O.D. Sampler				
<u>19.0</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS				Trace	Less than 5%		Cohesionless Density		Cohesive Consistency		
				Few	5 to 10%		0 - 10	Loose	0 - 4	Soft	
				Little	15 to 25%		10 - 30	Medium Dense	4 - 8	Medium Stiff	
				Some	30 to 45%		30 - 50	Dense	8 - 15	Stiff	
				Mostly	50 to 100%		50 +	Very Dense	15 - 30	Very Stiff	
									30 +	Hard	
LOCATION OF BORING See Boring Location Plan											
DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*	SOIL IDENTIFICATION Remarks include color, type of soil, etc. Rock-color, type, condition, hardness		
				0-6	6-12	12-18					
	--	0.0-1.5	SS	3	3	3	Damp	0.7	Topsoil		
									Brown to Brown Mottled Gray Lean Clay (CL) - moderate plasticity, few sand, trace gravel and cobbles		
	--	2.0-2.5	SS	50			Moist				
								4.0	Brown Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel		
5	4.0	4.0-5.5	SS	6	5	4	Moist to Very Moist				
	--	8.5-10.0	SS	8	8	8	Moist				
	--	13.5-15.0	SS	6	8	11	Moist	13.0			
									Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel		
									Water Seepage at 17 feet		
									Gray Silty Sand with Gravel (SM) - mostly fine to coarse sand, little silt, little gravel		
	--	18.5-20.0	SS	7	14	19	Wet	17.0			
									BOTTOM OF BORING: 20 feet		
								20.0			

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-14

CLIENT K2 Retail Construction and Development PROJ. SURF. ELEV. _____
 NO. 25-G-30380 DATE DRILLED 8/6/2025

GROUND WATER OBSERVATION <u>None</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Proportions Used Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	140 lb Wt. x 30" fall on 2" O.D. Sampler <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 33%;">Cohesionless Density</td> <td style="width: 33%;"></td> <td style="width: 33%;">Cohesive Consistency</td> </tr> <tr> <td>0 - 10 Loose</td> <td>0 - 4</td> <td>Soft</td> </tr> <tr> <td>10 - 30 Medium Dense</td> <td>4 - 8</td> <td>Medium Stiff</td> </tr> <tr> <td>30 - 50 Dense</td> <td>8 - 15</td> <td>Stiff</td> </tr> <tr> <td>50 + Very Dense</td> <td>15 - 30</td> <td>Very Stiff</td> </tr> <tr> <td></td> <td>30 +</td> <td>Hard</td> </tr> </table>	Cohesionless Density		Cohesive Consistency	0 - 10 Loose	0 - 4	Soft	10 - 30 Medium Dense	4 - 8	Medium Stiff	30 - 50 Dense	8 - 15	Stiff	50 + Very Dense	15 - 30	Very Stiff		30 +	Hard
Cohesionless Density		Cohesive Consistency																		
0 - 10 Loose	0 - 4	Soft																		
10 - 30 Medium Dense	4 - 8	Medium Stiff																		
30 - 50 Dense	8 - 15	Stiff																		
50 + Very Dense	15 - 30	Very Stiff																		
	30 +	Hard																		

LOCATION OF BORING See Boring Location Plan

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION					
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness					
				0-6	6-12	12-18									
	--	0.0-1.5	SS	2	3	2	Damp	0.9	Topsoil						
									Brown to Brown Mottled Gray Lean Clay with Sand (CL) - moderate plasticity, few to little sand, trace gravel						
	4.0	2.0-3.5	SS	3	2	2	Moist								
	4.0	4.0-5.5	SS	3	3	4	Moist								
5								6.5	Brown Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel						
	4.5+	8.5-10.0	SS	15	30	43	Moist								
10															
								12.0	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel; frequent sand and silt layers						
	4.5+	13.5-15.0	SS	9	13	16	Moist to Very Moist								
15															
	--	18.5-20.0	SS	15	19	23	Very Moist								
								20.0	BOTTOM OF BORING: 20 feet						

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-16
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/12/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
None FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION						
				From	To	12-18				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness						
				0-6	6-12	12-18										
	--	0.0-1.5	SS	2	2	3	Damp	0.7	▲▲▲▲▲▲▲▲▲▲	Topsoil						
									▨▨▨▨▨▨▨▨▨▨	Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand						
	2.0	2.0-3.5	SS	2	3	3	Moist to Very Moist									
								4.0	▨▨▨▨▨▨▨▨▨▨							
	3.0	4.0-5.5	SS	4	4	4	Moist			Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel						
5									▨▨▨▨▨▨▨▨▨▨							
	4.5+	8.5-10.0	SS	3	4	7	Moist	9.0	▨▨▨▨▨▨▨▨▨▨	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel; frequent sand and silt layers						
10									▨▨▨▨▨▨▨▨▨▨							
	4.5+	13.5-15.0	SS	10	11	12	Moist	15.0	▨▨▨▨▨▨▨▨▨▨							
15									▨▨▨▨▨▨▨▨▨▨							

										BOTTOM OF BORING: 15 feet						

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-18

CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____ DATE DRILLED 8/6/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
<u>8.0</u> FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION						
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness						
				0-6	6-12	12-18										
	--	0.0-1.5	SS	1	1	2	Damp	0.7	▲▲▲▲▲	Topsoil						
									▨▨▨▨▨	Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand						
	3.0	2.0-3.5	SS	2	2	2	Moist									
	4.0	4.0-5.5	SS	2	3	2	Moist	5.0								
5									▨▨▨▨▨	Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel						
	--	8.5-10.0	SS	5	7	6	Moist to Very Moist			▨▨▨▨▨	Water Seepage at 9 feet					
10																
								12.0								
									▨▨▨▨▨	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel						
	4.5+	13.5-15.0	SS	12	12	14	Moist									
15								15.0								
										-----	BOTTOM OF BORING: 15 feet					

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. B-19

CLIENT K2 Retail Construction and Development PROJ. SURF. ELEV. _____
 NO. 25-G-30380 DATE DRILLED 8/6/2025

GROUND WATER OBSERVATION <u>None</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Proportions Used Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	140 lb Wt. x 30" fall on 2" O.D. Sampler <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="border-right: 1px solid black;">Cohesionless Density</td> <td>Cohesive Consistency</td> </tr> <tr> <td style="border-right: 1px solid black;">0 - 10 Loose</td> <td>0 - 4 Soft</td> </tr> <tr> <td style="border-right: 1px solid black;">10 - 30 Medium Dense</td> <td>4 - 8 Medium Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">30 - 50 Dense</td> <td>8 - 15 Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">50 + Very Dense</td> <td>15 - 30 Very Stiff</td> </tr> <tr> <td></td> <td>30 + Hard</td> </tr> </table>	Cohesionless Density	Cohesive Consistency	0 - 10 Loose	0 - 4 Soft	10 - 30 Medium Dense	4 - 8 Medium Stiff	30 - 50 Dense	8 - 15 Stiff	50 + Very Dense	15 - 30 Very Stiff		30 + Hard
Cohesionless Density	Cohesive Consistency													
0 - 10 Loose	0 - 4 Soft													
10 - 30 Medium Dense	4 - 8 Medium Stiff													
30 - 50 Dense	8 - 15 Stiff													
50 + Very Dense	15 - 30 Very Stiff													
	30 + Hard													

LOCATION OF BORING See Boring Location Plan

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*	SOIL IDENTIFICATION Remarks include color, type of soil, etc. Rock-color, type, condition, hardness
				0-6	6-12	12-18			
	--	0.0-1.5	SS	3	3	3	Damp	0.6	Topsoil
									Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand
	2.0	2.0-3.5	SS	4	3	3	Moist		
									Gray fat clay (CH) veining
	--	4.0-5.5	SS	2	2	2	Very Moist	5.5	
									Brown Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel and cobble fragments
	3.0	8.5-10.0	SS	6	9	11	Moist to Very Moist		
									Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel
	3.0	13.5-15.0	SS	5	7	8	Moist to Very Moist	13.0	
								15.0	
----- BOTTOM OF BORING: 15 feet									

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. P-1
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/8/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
None FEET BELOW SURFACE AT COMPLETION	Trace Less than 5%	Cohesionless Density	Cohesive Consistency
_____ FEET BELOW SURFACE AT 24 HOURS	Few 5 to 10%	0 - 10 Loose	0 - 4 Soft
_____ FEET BELOW SURFACE AT _____ HOURS	Little 15 to 25%	10 - 30 Medium Dense	4 - 8 Medium Stiff
	Some 30 to 45%	30 - 50 Dense	8 - 15 Stiff
	Mostly 50 to 100%	50 + Very Dense	15 - 30 Very Stiff
			30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness
				0-6	6-12	12-18				
	--	0.0-1.5	SS	2	3	2	Damp		Topsoil	
								1.1	Stained to Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand	
	2.0	2.0-3.5	SS	5	3	4	Moist			
								4.0	Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel; thin sand and silt layers	
	4.0	4.0-5.5	SS	5	6	4	Moist			
5										
	4.5+	8.5-10.0	SS	9	11	9	Moist	9.0	Gray Sandy Lean Clay with Gravel (CL); glacial till - low to moderate plasticity, little sand, little gravel	
								10.0		
10										
									BOTTOM OF BORING: 10 feet	
15										

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. P-3
 CLIENT K2 Retail Construction and Development PROJ. SURF. ELEV. _____
 NO. 25-G-30380 DATE DRILLED 8/12/2025

GROUND WATER OBSERVATION <u>None</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Proportions Used Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	140 lb Wt. x 30" fall on 2" O.D. Sampler <table style="width: 100%; border-collapse: collapse;"> <tr> <td style="border-right: 1px solid black;">Cohesionless Density</td> <td>Cohesive Consistency</td> </tr> <tr> <td style="border-right: 1px solid black;">0 - 10 Loose</td> <td>0 - 4 Soft</td> </tr> <tr> <td style="border-right: 1px solid black;">10 - 30 Medium Dense</td> <td>4 - 8 Medium Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">30 - 50 Dense</td> <td>8 - 15 Stiff</td> </tr> <tr> <td style="border-right: 1px solid black;">50 + Very Dense</td> <td>15 - 30 Very Stiff</td> </tr> <tr> <td></td> <td>30 + Hard</td> </tr> </table>	Cohesionless Density	Cohesive Consistency	0 - 10 Loose	0 - 4 Soft	10 - 30 Medium Dense	4 - 8 Medium Stiff	30 - 50 Dense	8 - 15 Stiff	50 + Very Dense	15 - 30 Very Stiff		30 + Hard
Cohesionless Density	Cohesive Consistency													
0 - 10 Loose	0 - 4 Soft													
10 - 30 Medium Dense	4 - 8 Medium Stiff													
30 - 50 Dense	8 - 15 Stiff													
50 + Very Dense	15 - 30 Very Stiff													
	30 + Hard													

LOCATION OF BORING See Boring Location Plan

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION					
				From	To	To				Remarks include color, type of soil, etc. Rock-color, type, condition, hardness					
				0-6	6-12	12-18									
	3.0	0.0-1.5	SS	3	3	4	Moist	0.8	Topsoil						
									Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand						
	2.0	2.0-3.5	SS	3	2	2	Moist								
	1.0	4.0-5.5	SS	2	2	2	Very Moist								
5									Gray fat clay (CH) veining						
								7.0							
									Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel						
	4.5+	8.5-10.0	SS	7	8	11	Moist								
								10.0							
10									BOTTOM OF BORING: 10 feet						
15															

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



TEST BORING LOG

PROJECT NAME Gateway Crossing - Weldon and Faulk Roads - Jerome Twp., Ohio BORING NO. P-4
 CLIENT K2 Retail Construction and Development PROJ. NO. 25-G-30380 SURF. ELEV. _____
 DATE DRILLED 8/6/2025

GROUND WATER OBSERVATION	Proportions Used	140 lb Wt. x 30" fall on 2" O.D. Sampler	
<u>None</u> FEET BELOW SURFACE AT COMPLETION _____ FEET BELOW SURFACE AT 24 HOURS _____ FEET BELOW SURFACE AT _____ HOURS	Trace Less than 5% Few 5 to 10% Little 15 to 25% Some 30 to 45% Mostly 50 to 100%	Cohesionless Density	Cohesive Consistency
		0 - 10 Loose 10 - 30 Medium Dense 30 - 50 Dense 50 + Very Dense	0 - 4 Soft 4 - 8 Medium Stiff 8 - 15 Stiff 15 - 30 Very Stiff 30 + Hard

LOCATION OF BORING **See Boring Location Plan**

DEPTH	Pocket Penetrometer (tsf)	Sample Depths From To	Type of Sample	Blows per 6" on Sampler			Moisture Density or Consist.	Strata Change Depth*		SOIL IDENTIFICATION
				From	To	0-6				6-12
	--	0.0-1.5	SS	2	2	3	Damp			Topsoil
								1.1		Black Fat Clay (CH) - high plasticity, trace sand
	--	2.0-3.5	SS	4	3	2	Damp to Moist			
								3.0		Brown Mottled Gray Lean Clay (CL) - moderate to high plasticity, few sand
	3.0	4.0-5.5	SS	2	3	4	Moist			Gray fat clay (CH) veining
5								7.0		Brown Sandy Lean Clay (CL); glacial till - low to moderate plasticity, little to some sand, few gravel
	4.5+	8.5-10.0	SS	5	6	9	Moist			
10								10.0		
										BOTTOM OF BORING: 10 feet
15										

* The stratification lines represent the approximate boundary between soil types and the transition may be gradual.



